Truss Structures

upper chord members

verticals

lower chord members

diagonals

(a)

gusset plate

weld

(b)

(c)

Truss Definitions and Details

$+M$

$+M$

$-M$ $-M$
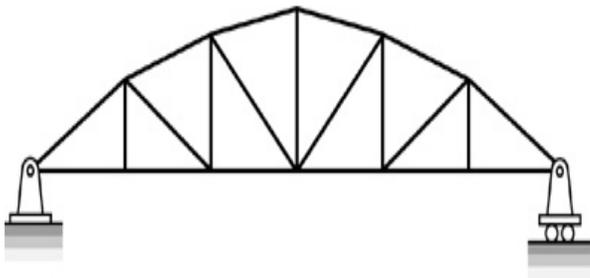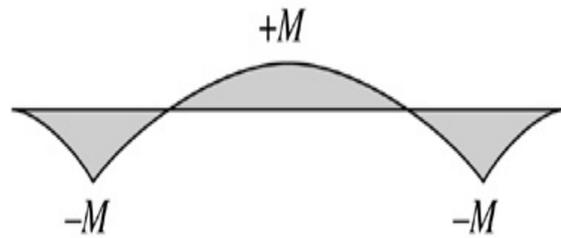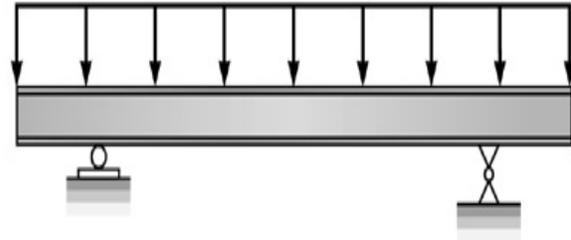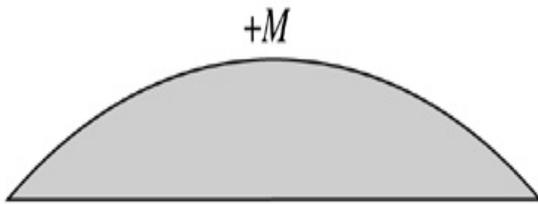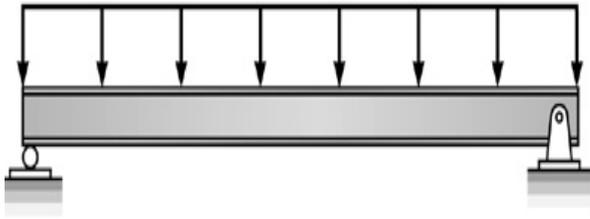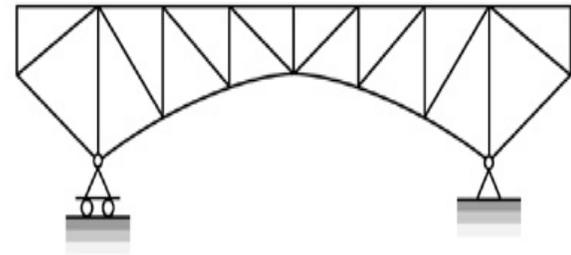
(a) (b)

Truss: Mimic Beam Behavior

2

transverse beam

truss

typical panel bracing

floor beam

truss

floor slab

stringer

(a)

diagonal bracing typical all panels

truss

floor beams
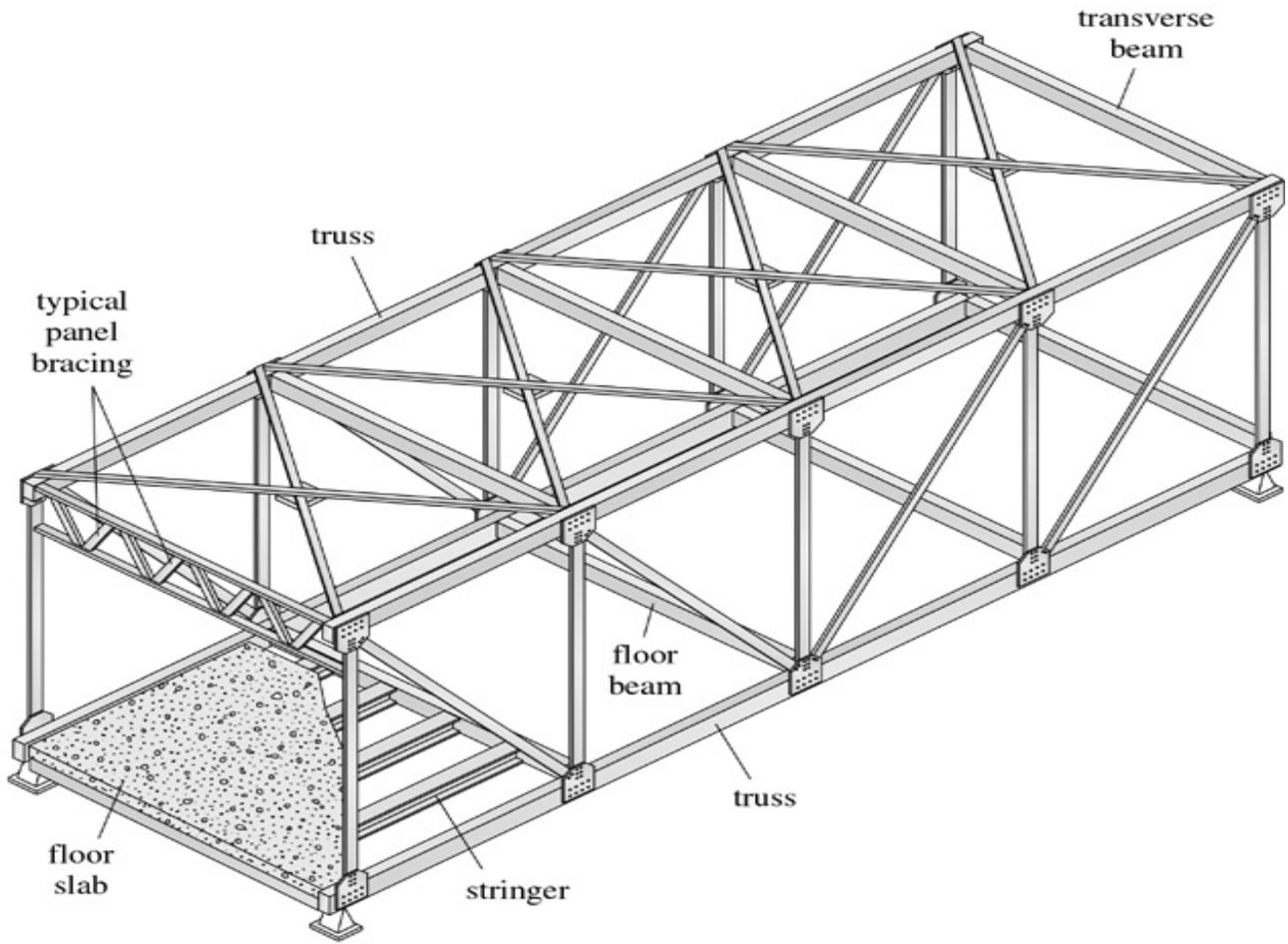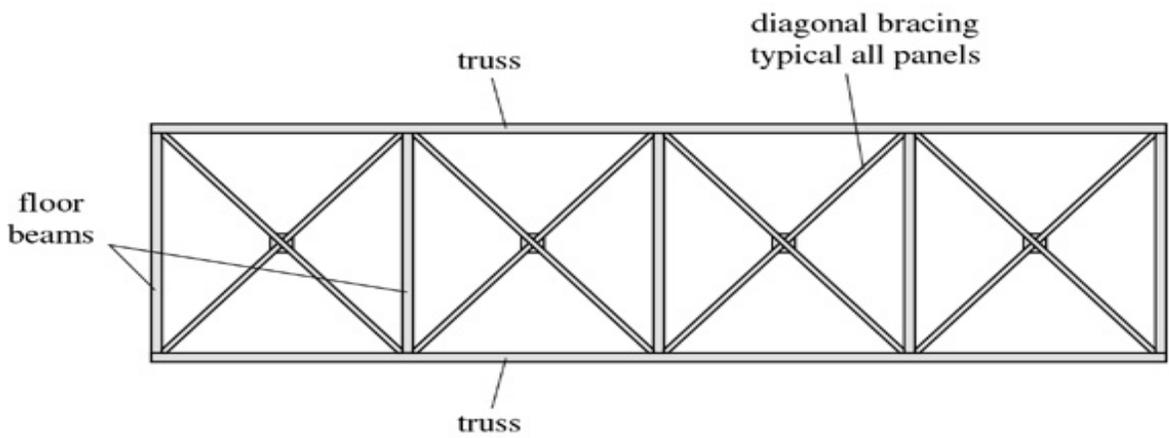
truss

(b)

Bridge Truss Details

3
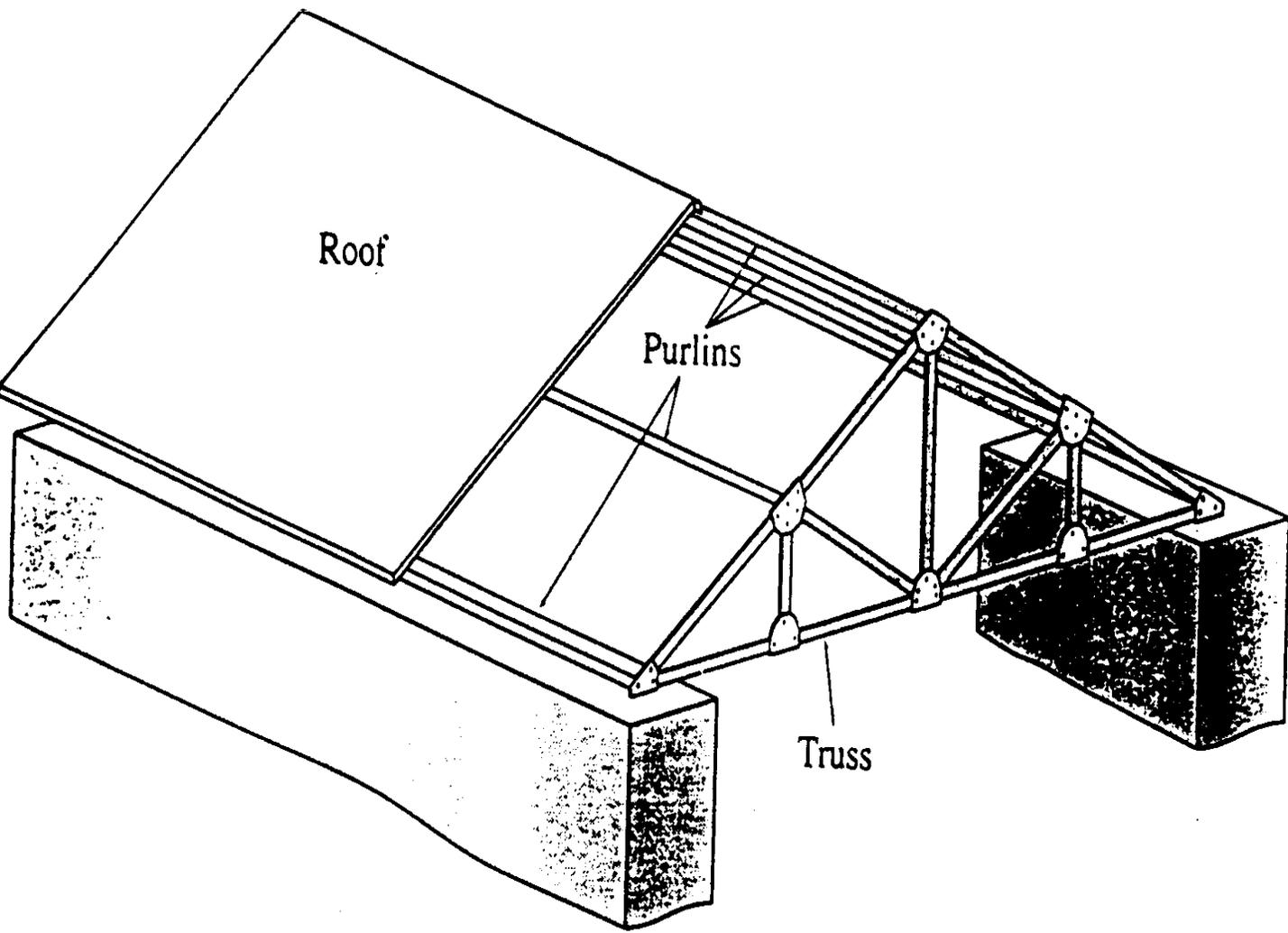
Framing of a Roof Supported Truss
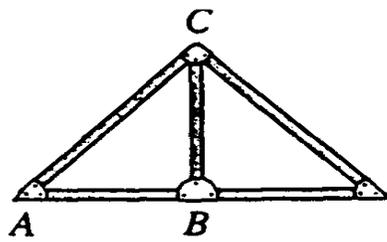
King post truss
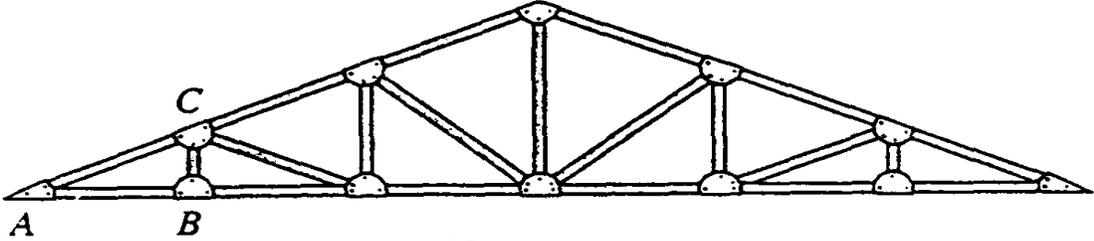
Howe truss
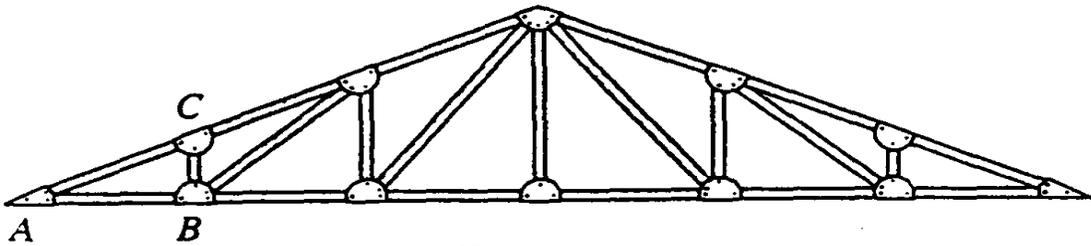
Pratt truss
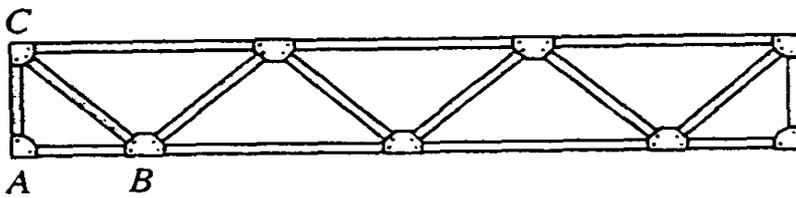
Warren truss
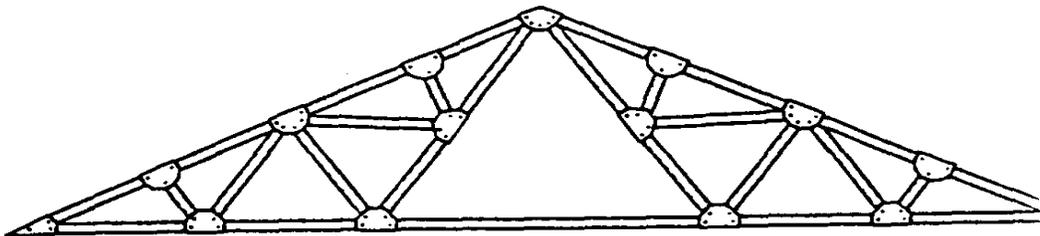
Fink truss

Common Roof Trusses

C

A   B

Howe truss

C

A   B

Pratt truss

C

A   B

Warren truss

C

A   B
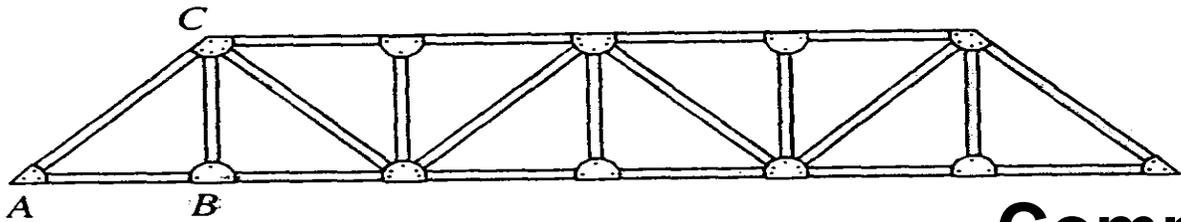
Parker truss

Common Bridge Trusses

C

A

B
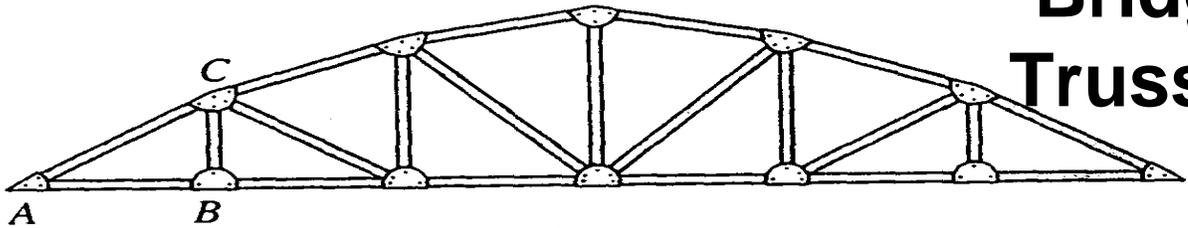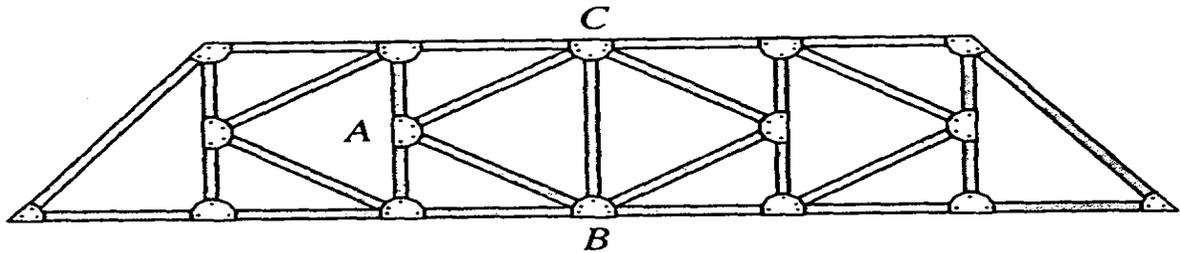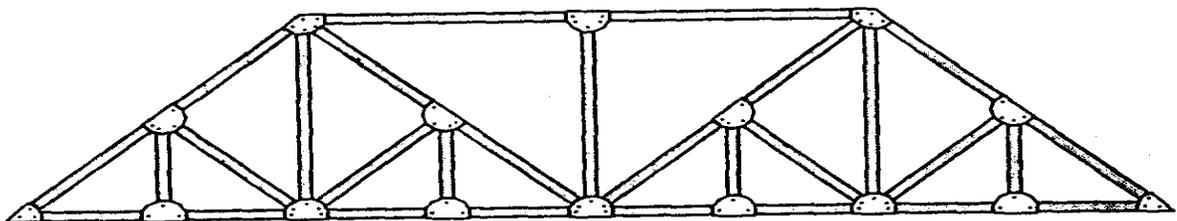
K truss

Baltimore truss

Buckling Calculations

$$P_{cr} = \frac{\pi^2 EI_{weak}}{(kL)^2}$$

$$= \text{buckling force}$$

$k = $ effective length factor

$k = 1$ for an ideal truss member

Types of Trusses

Basic Truss Element
    $\equiv$ three member triangular truss

Simple Trusses – composed of basic truss elements

$m = 3 + 2(j - 3) = 2j - 3$

for a simple truss

m $\equiv$ total number of members
 j $\equiv$ total number of joints
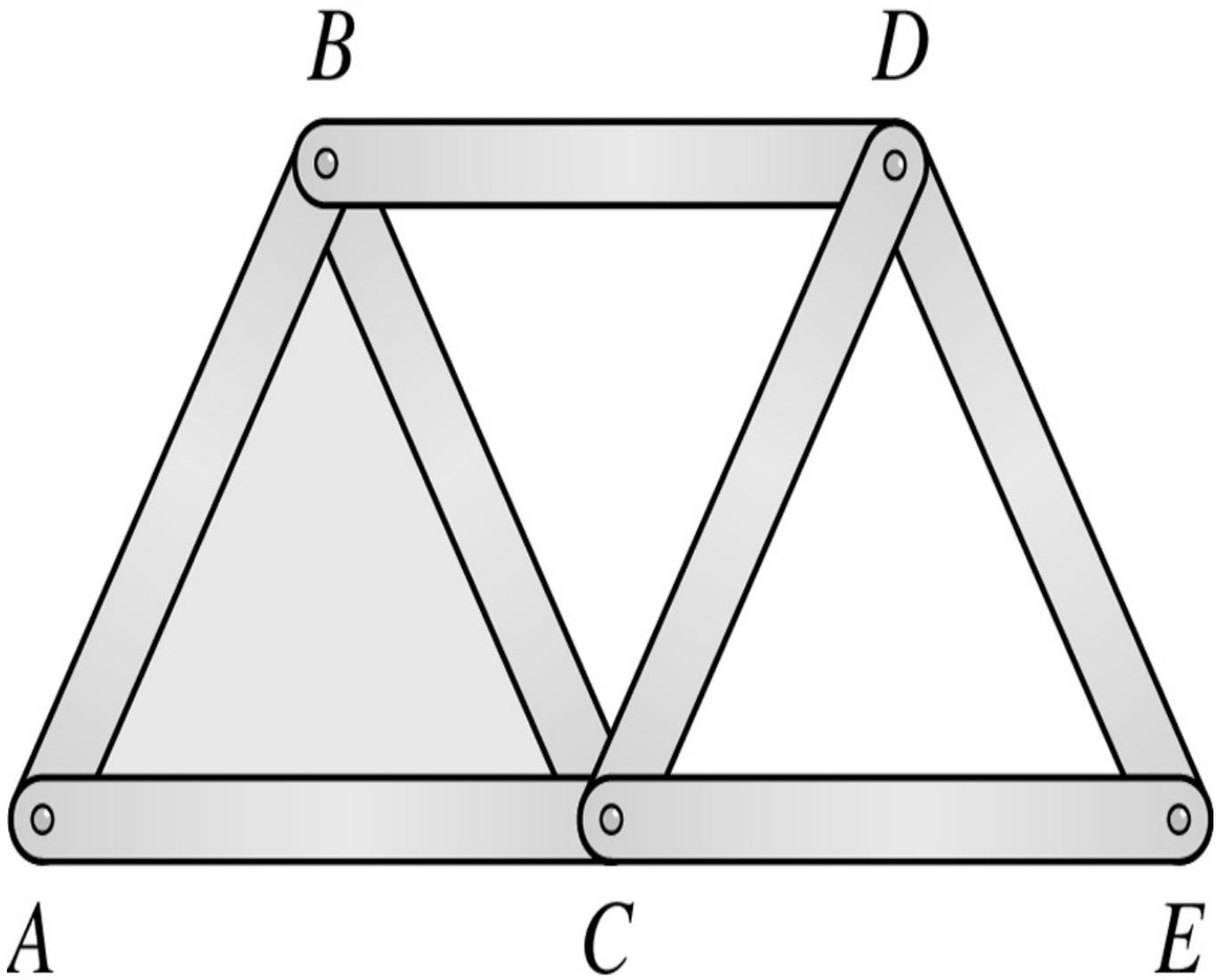
Simple Truss

Compound Trusses – constructed by connecting two or more simple trusses to form a single rigid body

simple truss
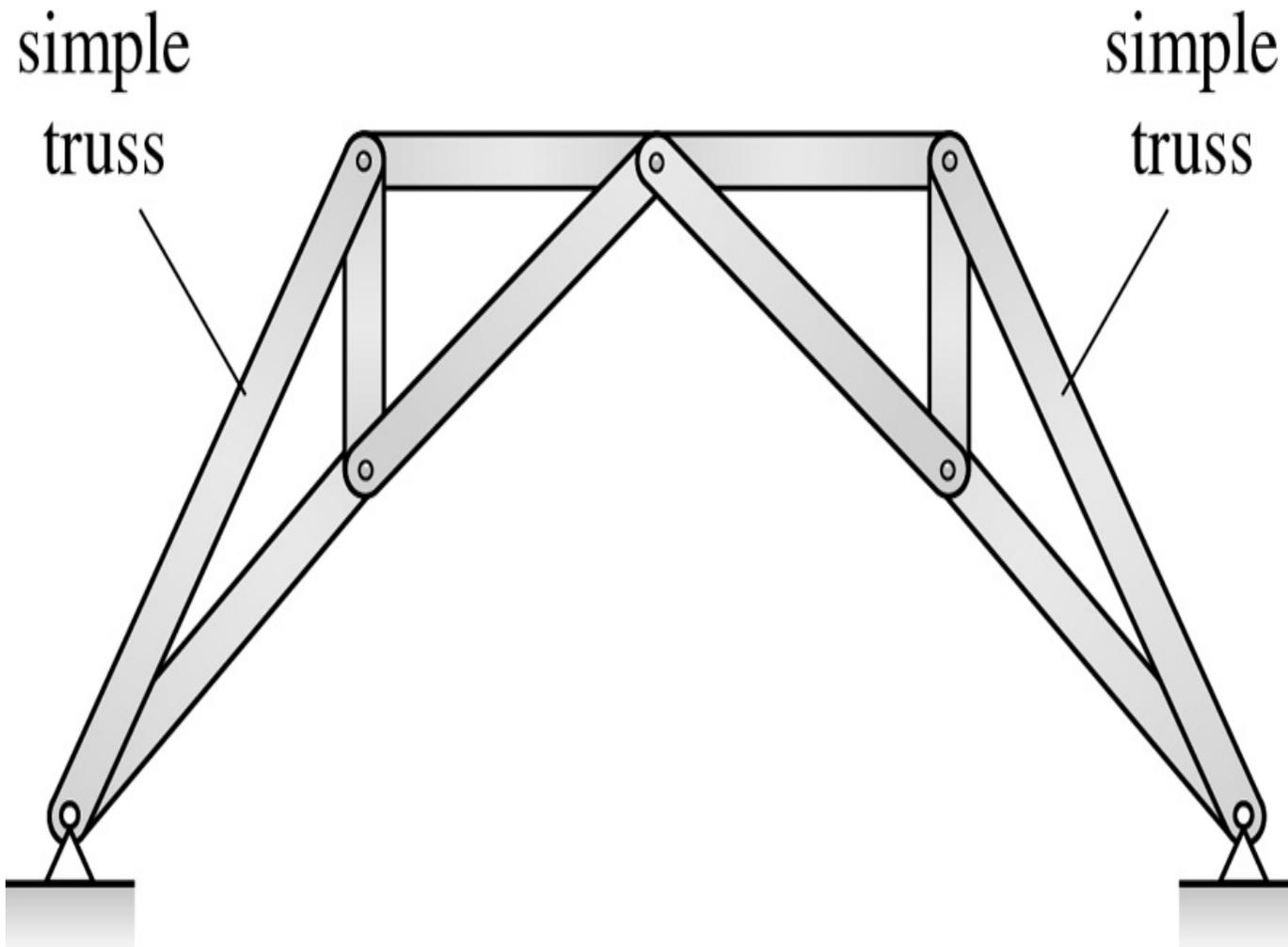
simple truss

Complex Trusses – truss that is neither simple nor compound
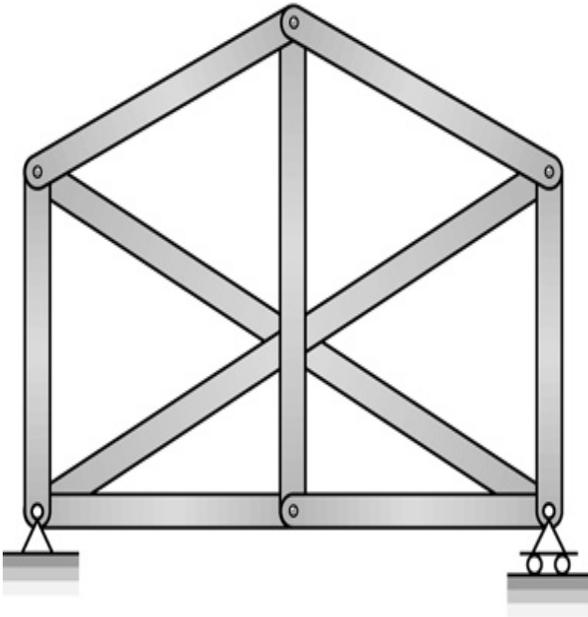
(a)
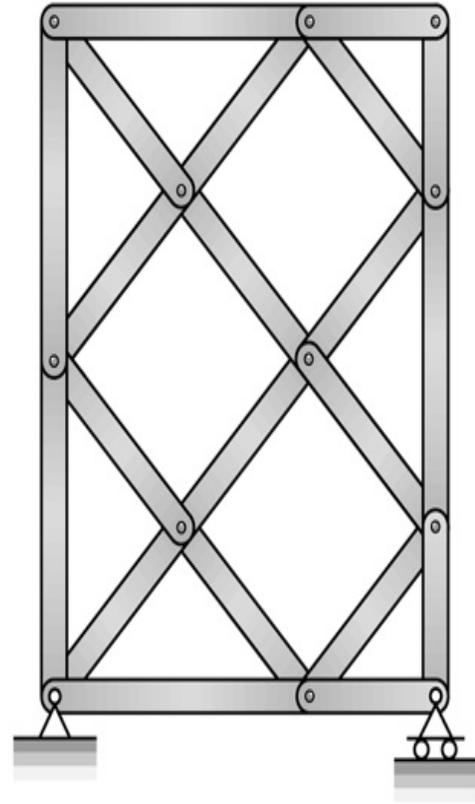
(b)

Analysis of Trusses

The analysis of trusses is usually based on the following simplifying assumptions:

- •The centroidal axis of each member coincides with the line connecting the centers of the adjacent members and the members only carry axial force.

- •All members are connected only at their ends by frictionless hinges in plane trusses.

- •All loads and support reactions are applied only at the joints.

The reason for making these assumptions is to obtain an ideal truss, i.e., a truss whose members are subjected only to axial forces.

Primary Forces $\equiv$ member axial forces determined from the analysis of an ideal truss

Secondary Forces $\equiv$ deviations from the idealized forces, i.e., shear and bending forces in a truss member.

Our focus will be on primary forces. If large secondary forces are anticipated, the truss should be analyzed as a frame.

Method of Joints

Method of Joints - the axial forces in the members of a statically determinate truss are determined by considering the equilibrium of its joints.

Tensile (**T**) **axial member force** is indicated on the joint by an arrow pulling away from the joint.

Compressive (**C**) **axial member force** is indicated by an arrow pushing toward the joint.

Method of Joints Example

22 kips

C

4
3

5

B

13

12

4

3

5

2

1

A

22 kips

D

6'

6'

11'   5'

24 kips   24 kips

(a)

Y_{AB} F_{AB}

X_{AB}

A

22 kips F_{AD}

24 kips

(b)

15

F_{BC}

y

x

B

X_{BD} Y_{BD}

40 kips

F_{BD}

(c)

X_{DC} ← F_{DC}

0

Y_{DC}

10 kips →

D

24 kips
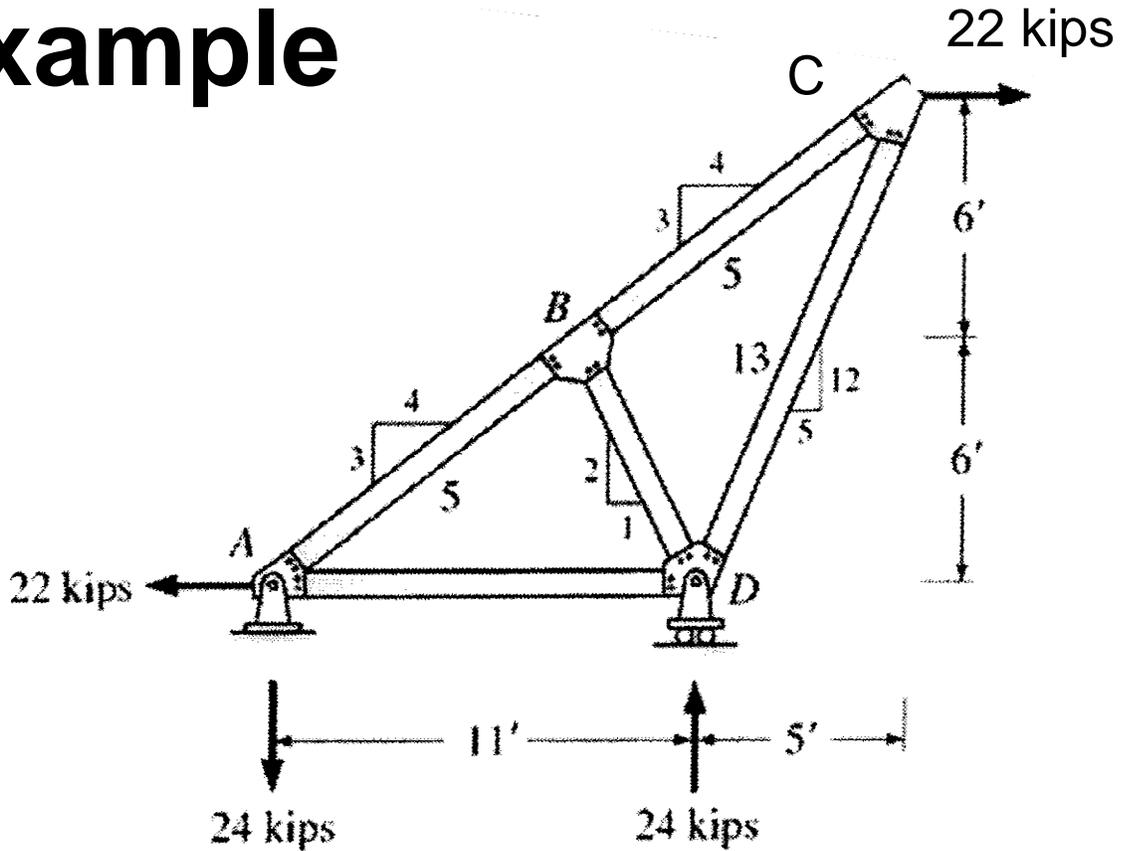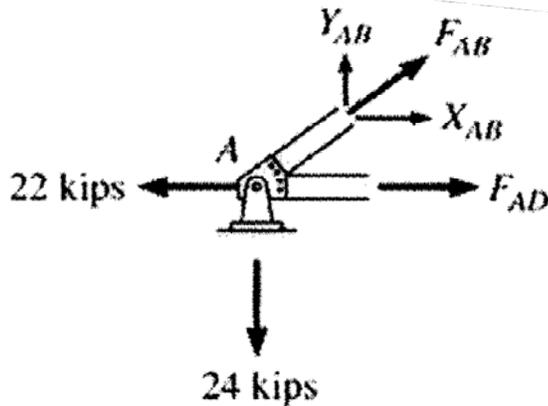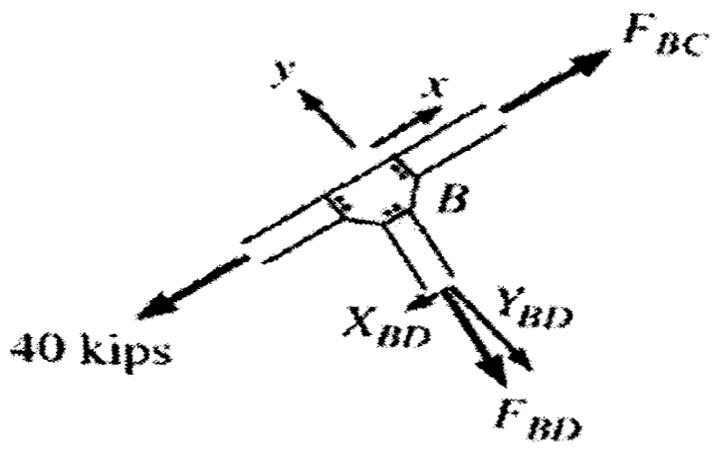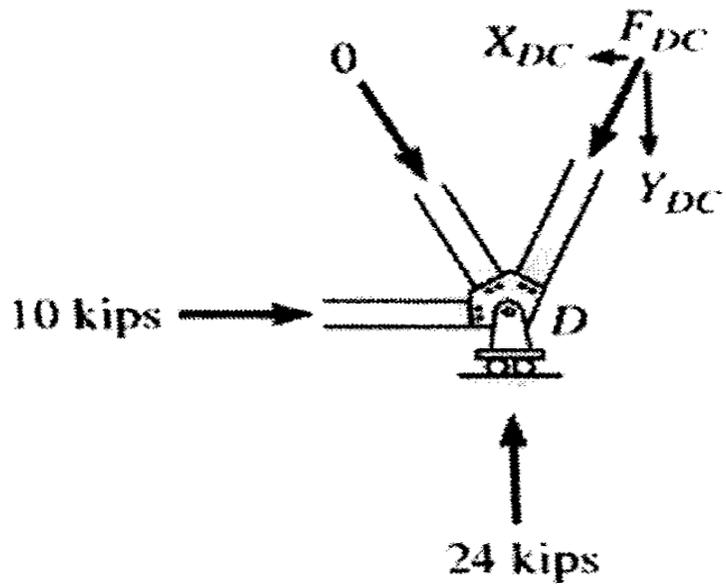
(d)
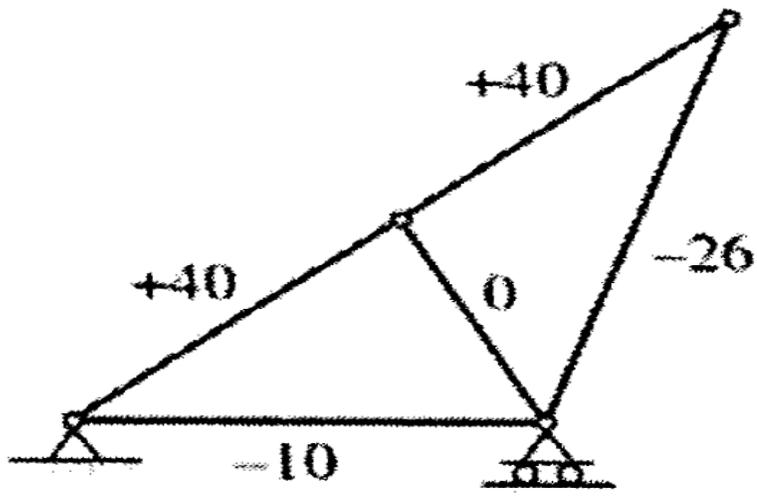
(e)

Truss Solution

Zero Force Members:

(a) If only two noncollinear members are connected to a joint that has no external loads or reactions applied to it, then the force in both members is zero.

(b) If three members, two of which are collinear, are connected to a joint that has no external loads or reactions applied to it, then the force in the member that is not collinear is zero.
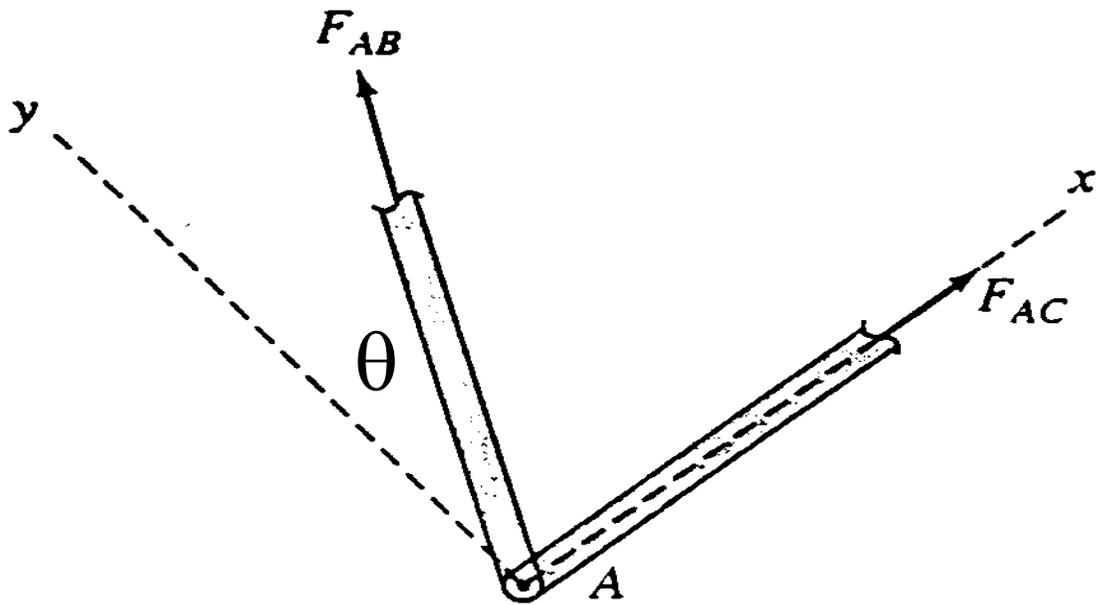
(a)
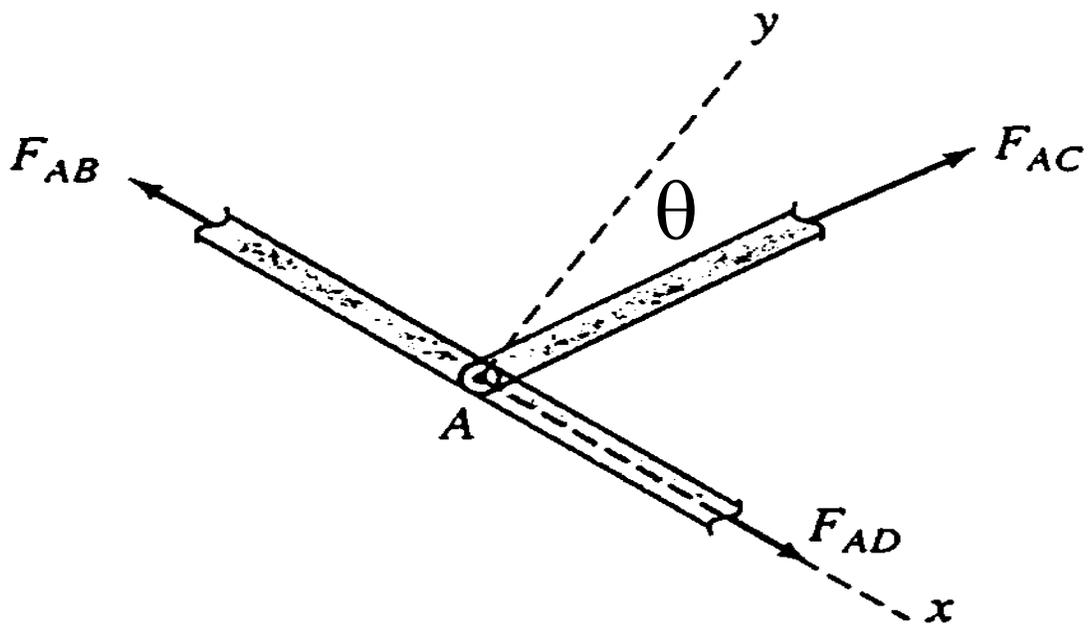
(b)

Zero Force Members

Zero Member Force Calculations

Figure (a):

$$\sum F_y = 0 = F_{AB} \cos \theta$$

$$\therefore F_{AB} = 0$$

$$\sum F_x = 0 = F_{AC} + F_{\!/\!\!AB}^{\,0} \sin \theta$$

$$\therefore F_{AC} = 0$$

Figure (b):

$$\sum F_y = 0 = F_{AC} \cos \theta$$

$$\therefore F_{AC} = 0$$

60 kips

0   D   0   E

C

B

0   0

0

A   0   M   0   L   K   F

0

0

J   0   G

0

0

I   0   H

180 kips   120 kips
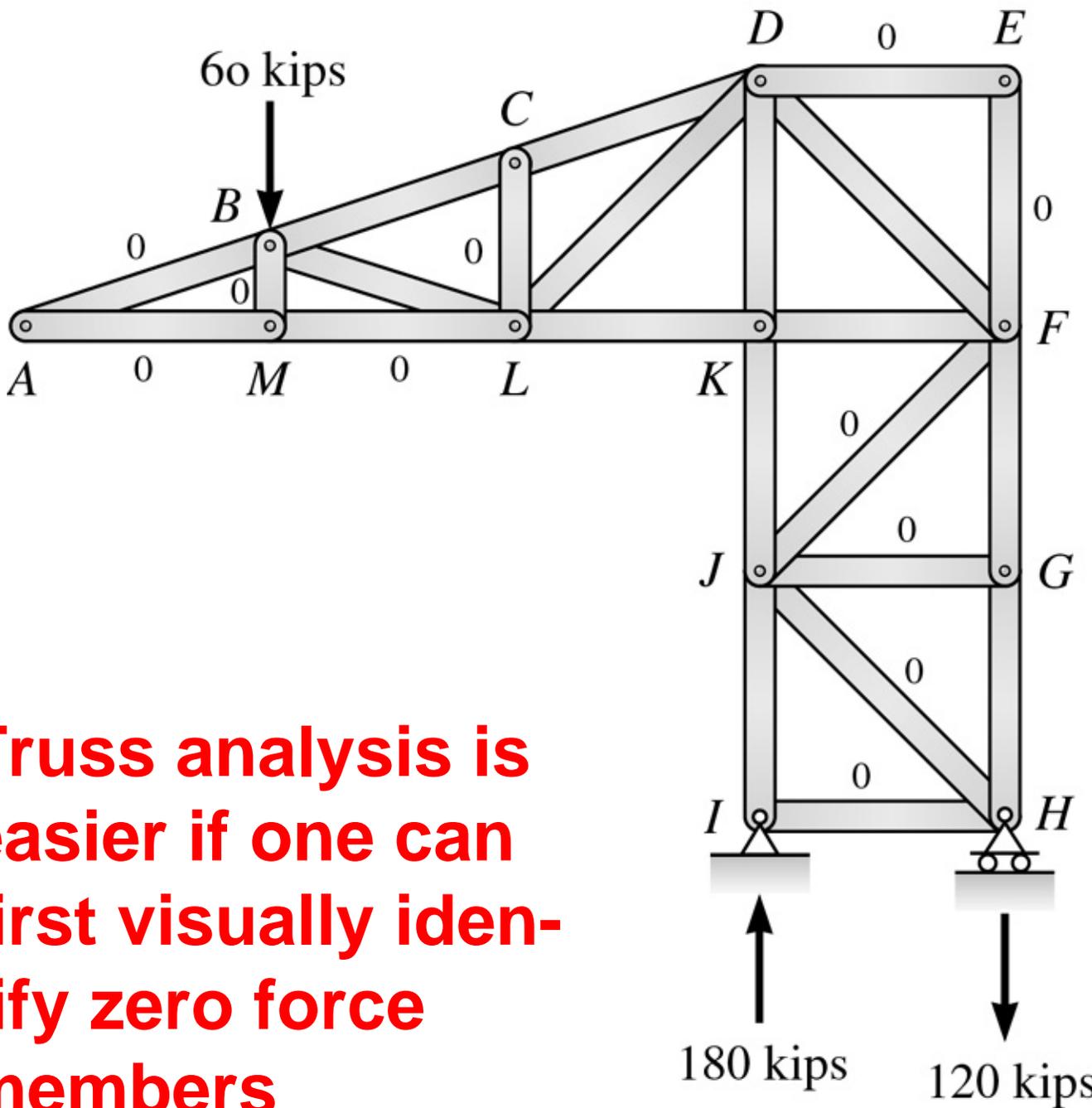
Truss analysis is easier if one can first visually iden- tify zero force members

21

Method of Sections

The method of sections enables one to determine forces in specific truss members directly.

Method of Sections $\equiv$ involves cutting the truss into two portions (free body diagrams, FBD) by passing an imaginary section through the members whose forces are desired. Desired member forces are determined by considering equilibrium of one of the two FBD of the truss.

Method of sections can be used to determine three unknown member forces per FBD since all three equilibrium equations can be used.
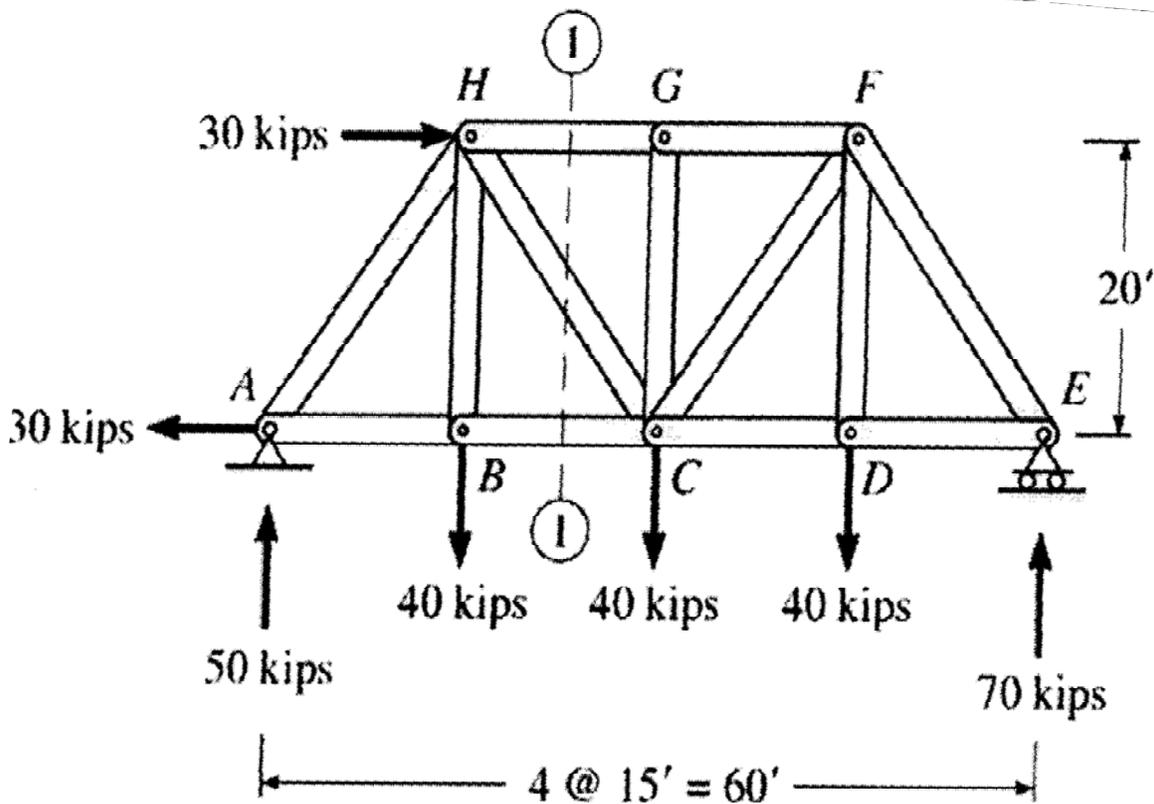
(a)

Method of Sections Example

H

30 kips ⟶ ⟵ F_{HG}

20'

X_{HC}

A

Y_{HC} F_{HC}

30 kips ⟵ F_{BC}

B

40 kips

50 kips

⊢— 15' —⊣

(b)

F_{BC} = _____

F_{HG} = _____

F_{HC} = _____
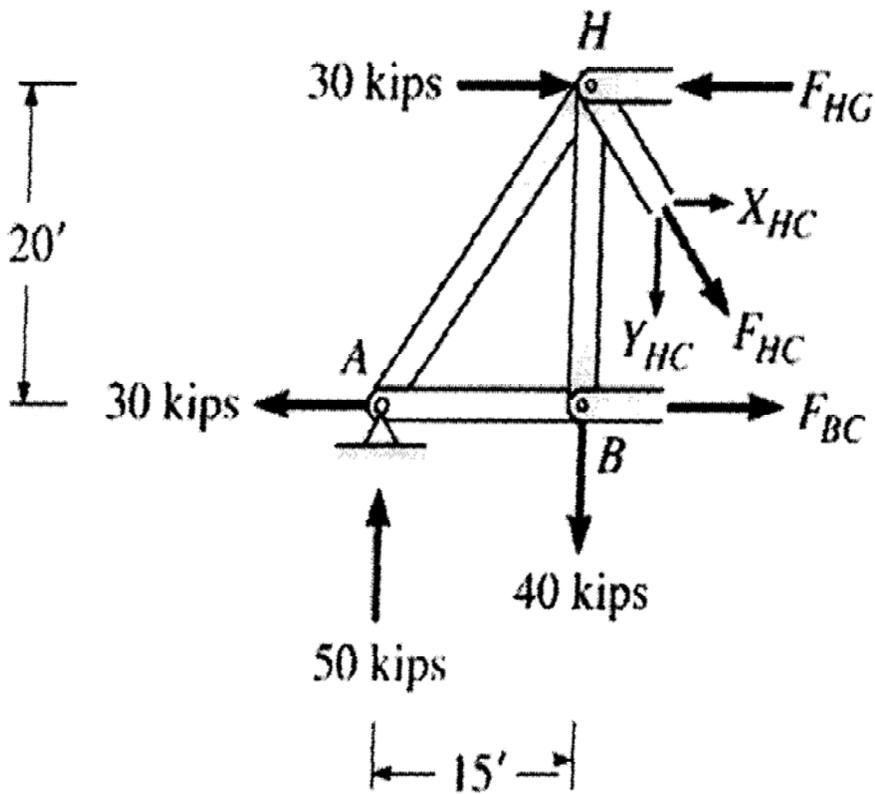
# Statics Principle of Transmissibility

(a)

(b)

(c)
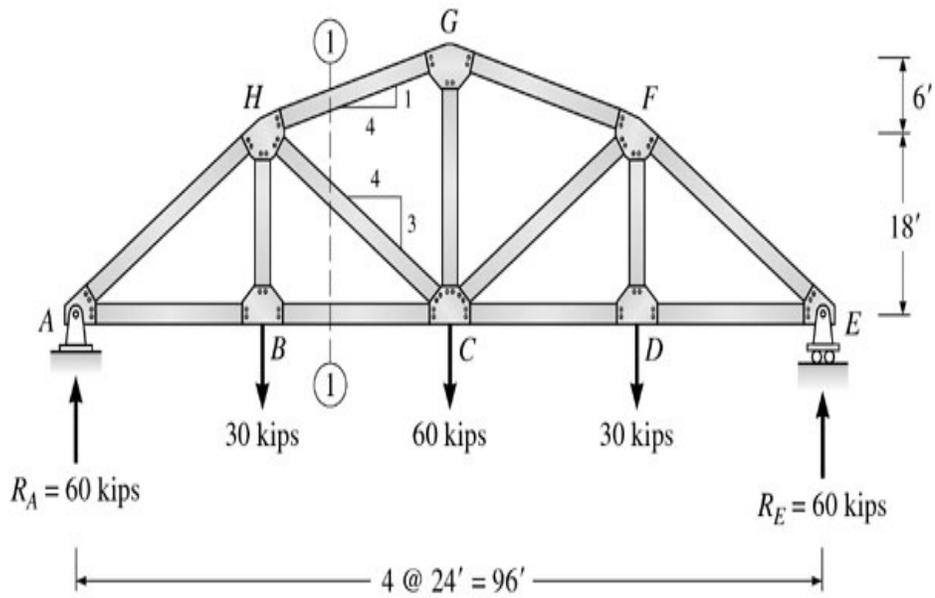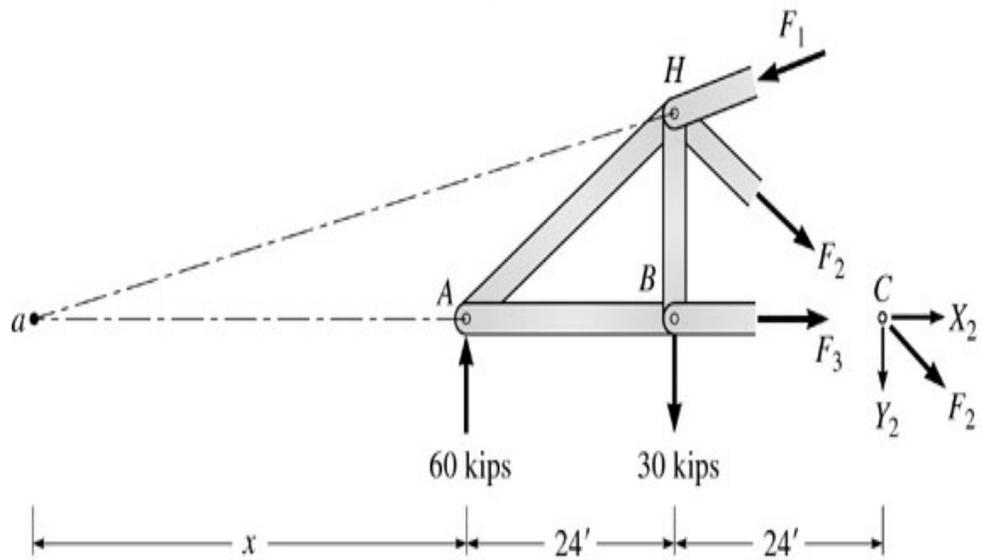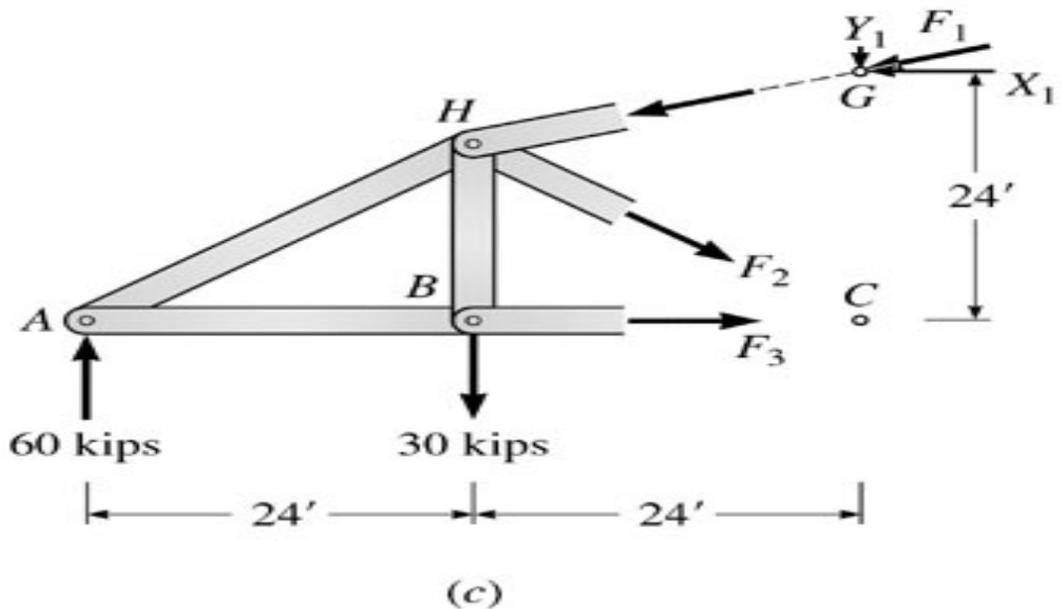
Transmissibility principle of statics states that a force can be applied at any point on its line of action without a change in the external effects

(a)

(b)

A

B

F_{BC}

F_{JB}

F_{JG}

G

F_{GF}

I

H

24 kips   48 kips

30'

20'

A   B   ① ②   C   D

15'

I   J   K

15'

H   G   ① ②   F   E

24 kips   48 kips   48 kips

20'   20'   20'

$$F_{BC} = \underline{\qquad}$$

$$F_{GF} = \underline{\qquad}$$

27

(c)

$$F_{JC} = \underline{\quad\quad}$$

$$F_{JF} = \underline{\quad\quad}$$
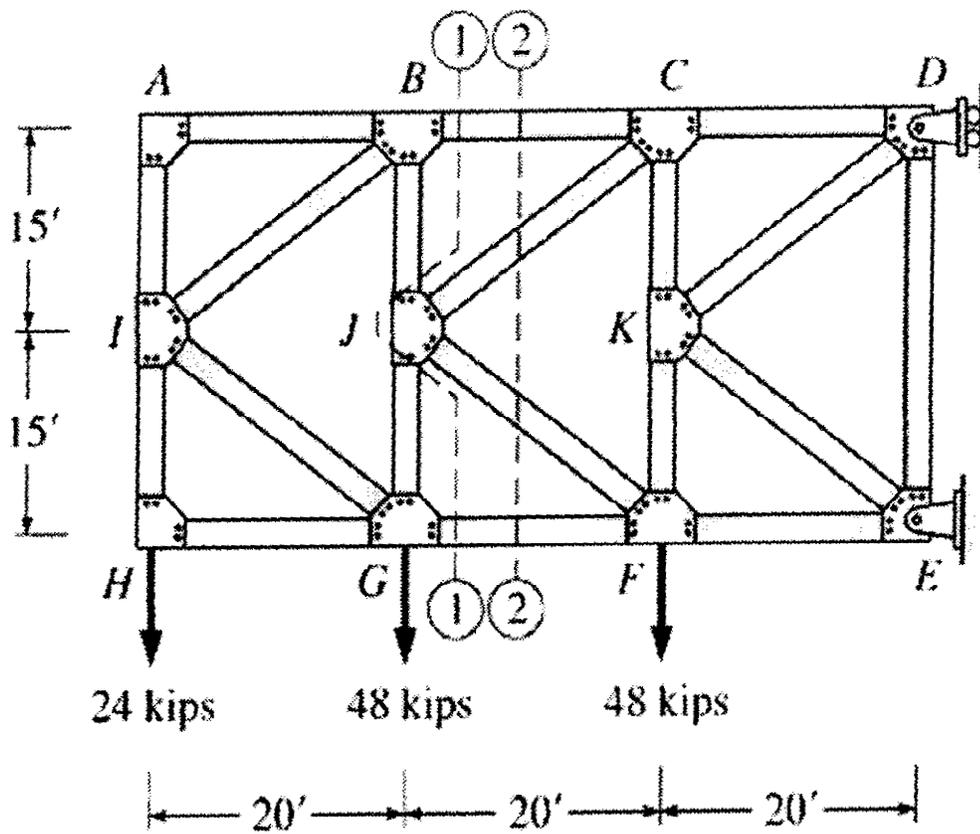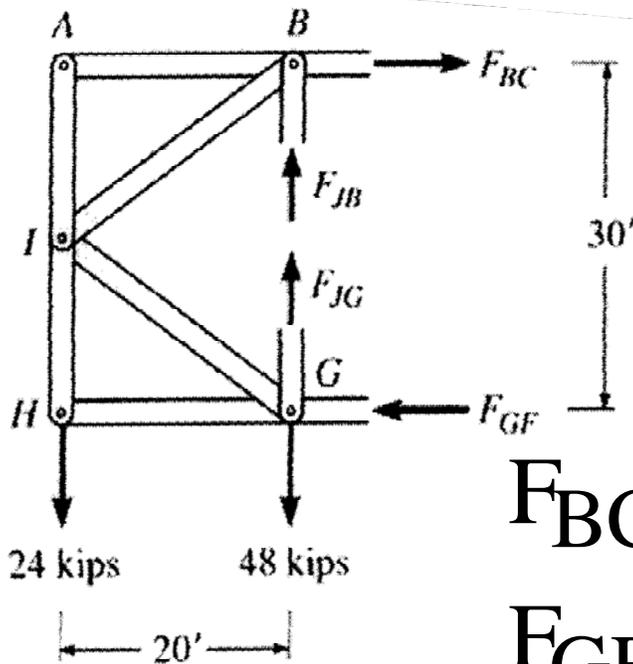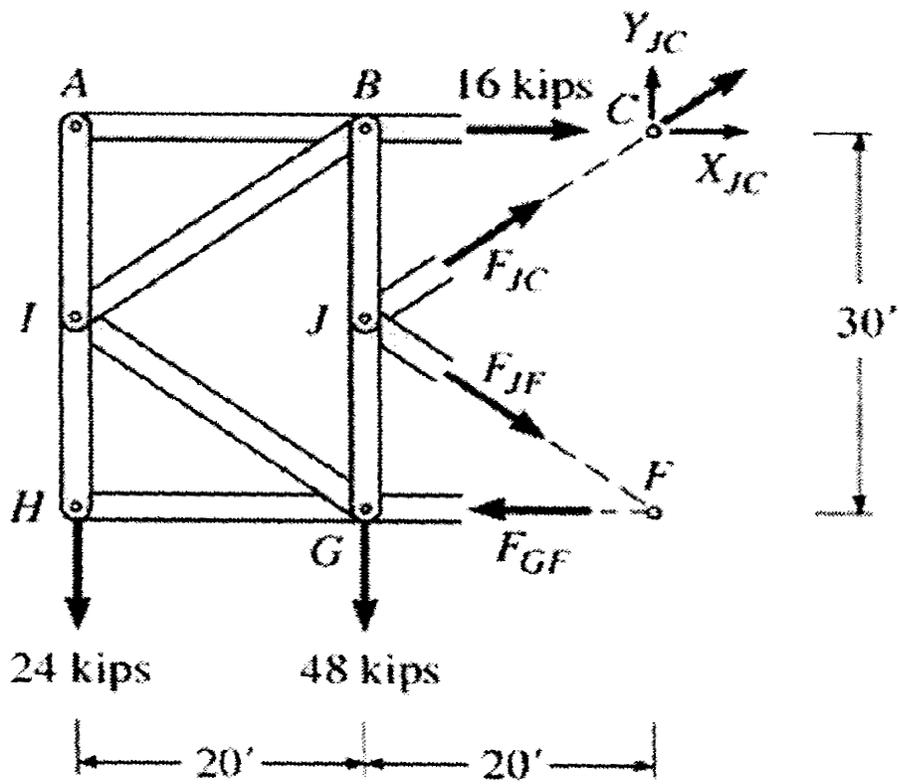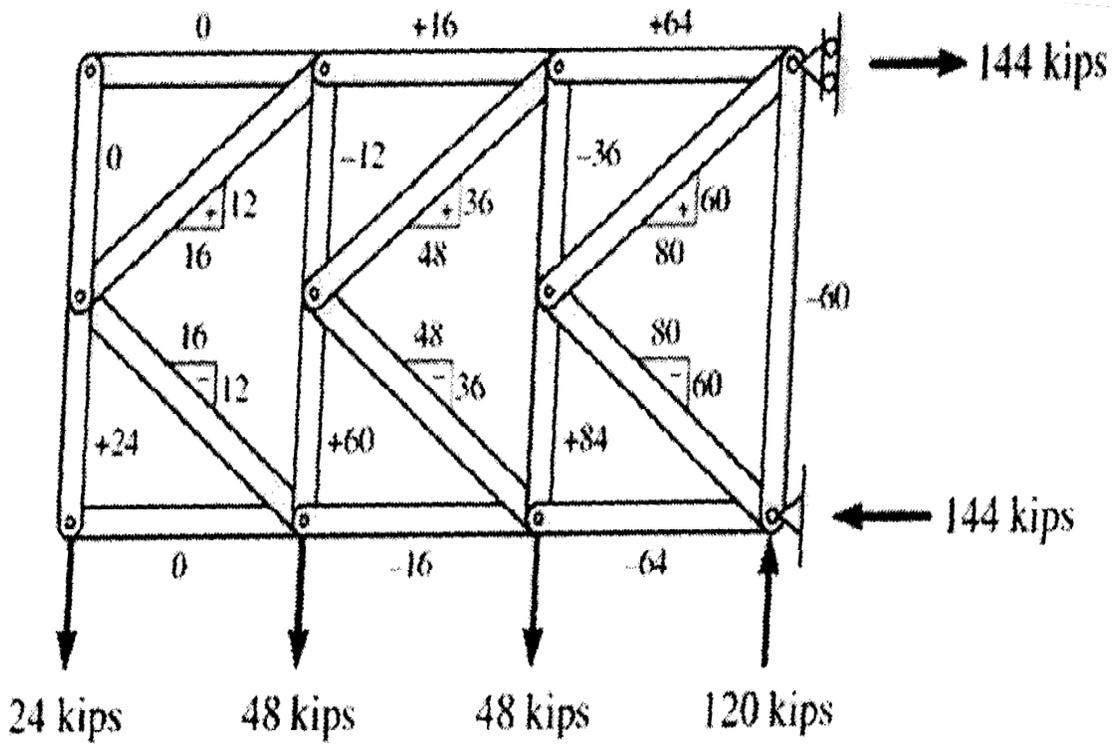
(d)

K-Truss Solution

Determinacy and Stability

Internal Stability $\equiv$ number and arrangement of members is such that the truss does not change its shape when detached from the supports.

External Instability $\equiv$ instability due to insufficient number or arrangement of external supports.

Internal Stability

m < 2j − 3
⇒ truss is internally unstable

m ≥ 2j − 3
⇒ truss is internally stable provided it is geometrically stable

m ≡ total number of members

j ≡ total number of joints

Geometric stability in the second condition requires that the members be properly arranged.

Statically Determinate Truss ≡ if all the forces in all its members as well as all the external reactions can be determined by using the equations of equilibrium.

Statically Indeterminate Truss ≡ if all the forces in all its members as well as all the external reactions cannot be determined by using the equations of equilibrium.

External Indeterminacy ≡ excess number of support reactions

Internal Indeterminacy $\equiv$ excess number of members

Redundants $\equiv$ excess members or reactions

Number of redundants defines the degree of static indeterminacy **I**

Summary

m + R < 2j

$\Rightarrow$ statically unstable truss

m + R = 2j

$\Rightarrow$ statically determinate truss

m + R $\geq$ 2j

$\Rightarrow$ statically indeterminate truss

The first condition is always true.

But, the **last two conditions are true if and only if the truss is geometrically stable**.

The analysis of unstable trusses will always lead to inconsistent, indeterminant, or infinite results.
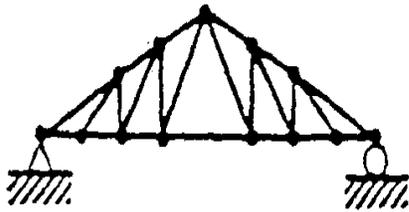
$m + r ? 2j$
$23 + 3 = 26$
Determinate
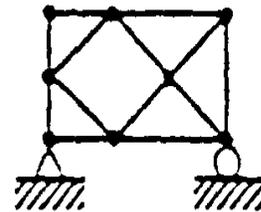
$m + r ? 2j$
$13 + 3 = 16$
Determinate
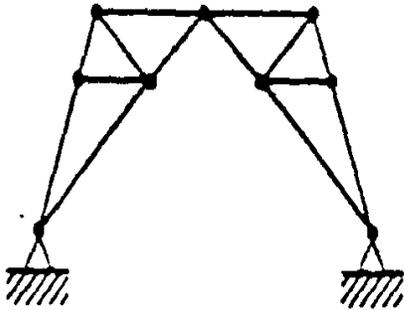
$m + r ? 2j$
$14 + 4 = 18$
Determinate
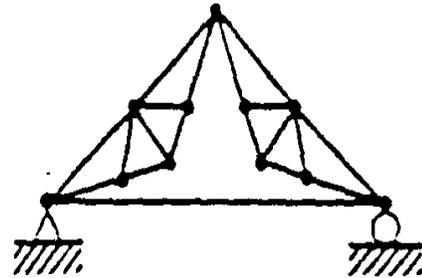
$m + r ? 2j$
$19 + 3 = 22$
Determinate
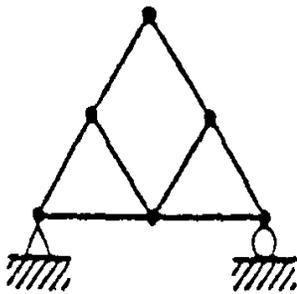
$m + r ? 2j$
$8 + 3 < 12$
Unstable
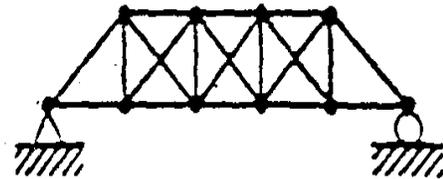
$m + r ? 2j$
$20 + 3 > 20$
3 Redundants
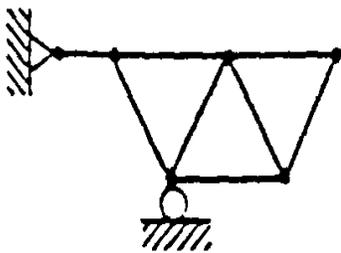
$m + r ? 2j$
$8 + 3 < 12$
Unstable
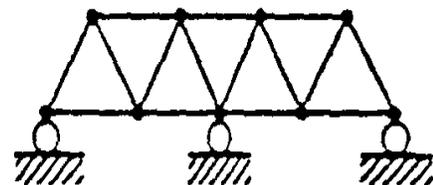
$m + r ? 2j$
$15 + 3 = 18$
Unstable

Truss Determinacy Calculations

35

(a)
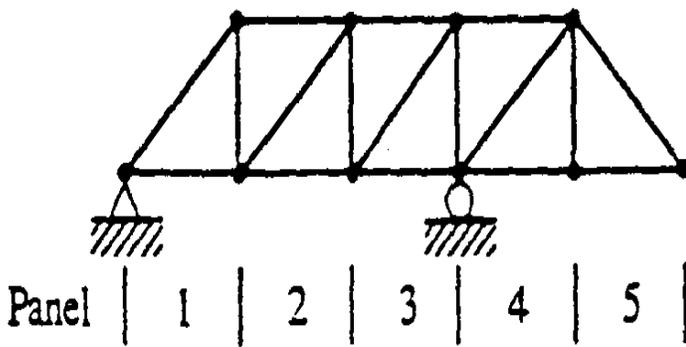
$m = 17$ $m + r = 2j$
$r = 3$ Statically
$j = 10$ determinate
            and stable

(b)

$m = 17$ $m + r = 2j$
$r = 3$ Unstable
$j = 10$

(c)
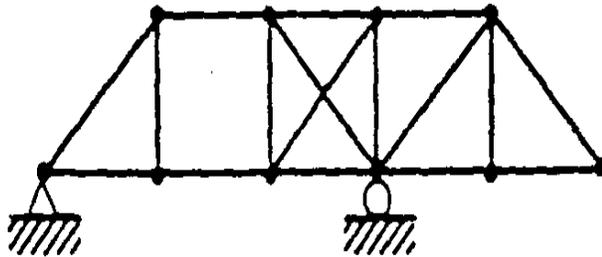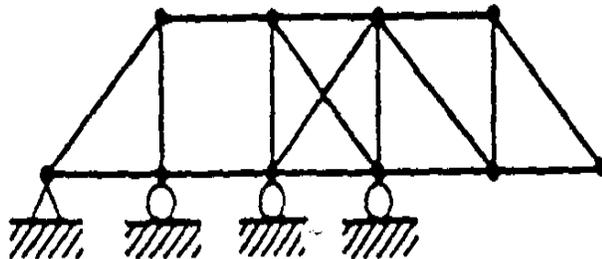
$m = 17$ $m + r > 2j$
$r = 5$ Stable
$j = 10$ 2 redundants

Truss Determinacy Calculations
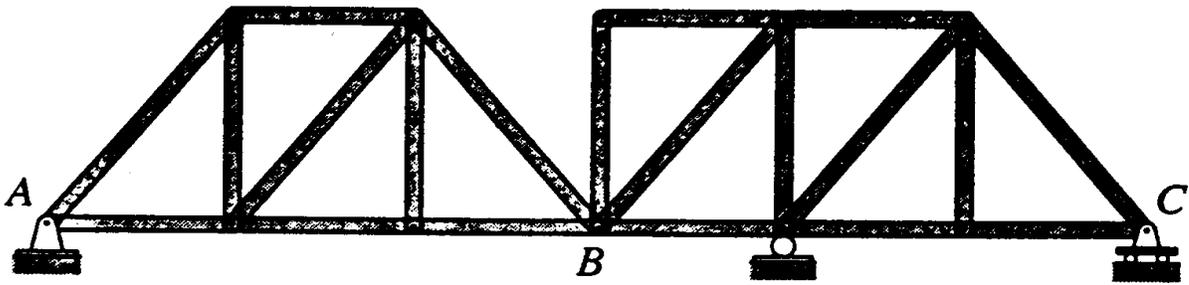
One equation of condition:

$$\Sigma M_B^{AB} = 0 \quad \text{or} \quad \Sigma M_B^{BC} = 0$$

(a)
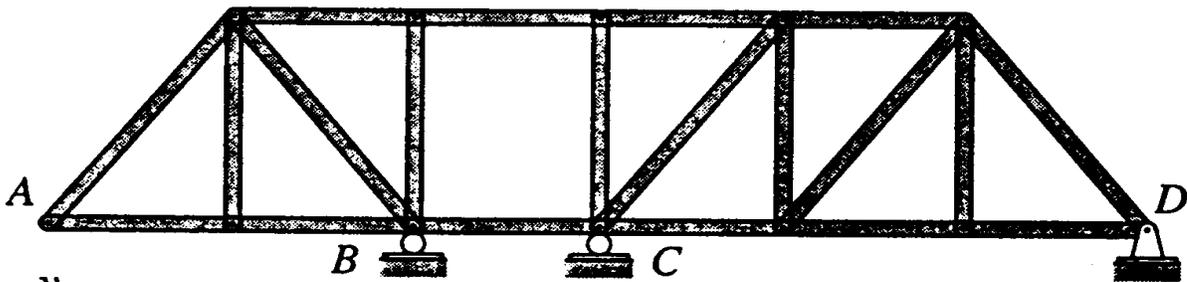
One equation of condition:

$$\Sigma F_y^{AB} = 0 \quad \text{or} \quad \Sigma F_y^{CD} = 0$$

(b)
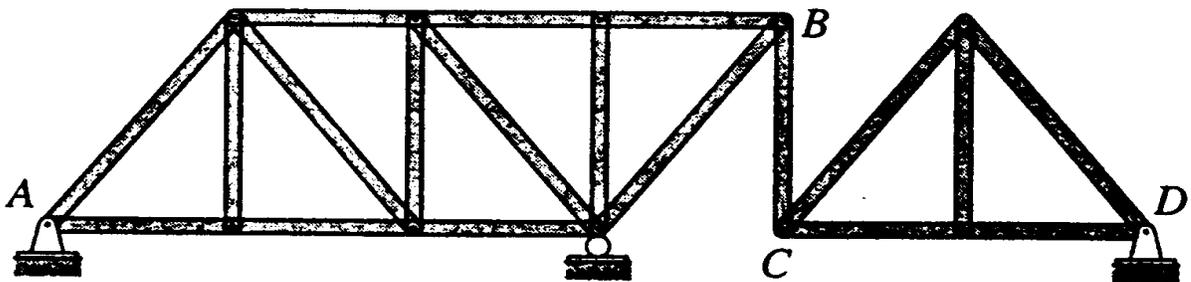
Two equations of condition:

$$\Sigma F_x^{AB} = 0 \quad \text{or} \quad \Sigma F_x^{CD} = 0$$

$$\Sigma M_B^{AB} = 0 \quad \text{or} \quad \Sigma M_C^{CD} = 0$$

(c)

Equations of Condition: Plane Trusses